

Design of Singly Reinforced Rectangular Beams:

$$S_b = \text{Max} \left(25, d_b, \frac{4}{3} d_{agg} \right) \quad \rightarrow \text{Minimum Bar Spacing, SBC304 – 18, 25.2.1}$$

$$S_l = \text{Max} \left(25, \frac{4}{3} d_{agg} \right) \quad \rightarrow \text{Minimum layer Spacing}$$

$$b_{min} = n d_b + (n - 1) S_b + 6 d_s - d_b + 2 \text{cover} \quad \rightarrow \text{minimum beam width to accomodate rebars}$$

$$n_{max} = \frac{b + S_b - 6 d_s + d_b - 2 \text{cover}}{d_b + S_b} \quad \rightarrow \text{maximum number of rebars in one layer}$$

$$R_n = \frac{M_u}{\phi b d^2} \quad \rightarrow \text{factor}$$

$$\rho = \frac{0.85 f'_c}{f_y} \left(1 - \sqrt{1 - \frac{4 R_n}{1.7 f'_c}} \right) \quad \rightarrow \text{steel ratio, for design purpose}$$

$$\rho = \frac{A_s}{b d} \quad \rightarrow \text{steel ratio}$$

$$\rho_{min} = \text{Max} \left(\frac{\sqrt{f'_c}}{4 f_y}, \frac{1.4}{f_y} \right) \quad \rightarrow \text{minimum steel ratio, SBC304 – 18, 9.6.1.2}$$

Design of a 300×600 mm rectangular section subjected to an ultimate bending moment $M_u = 300$ kN.m. given: $f'_c = 25$ MPa, $f_y = 420$ MPa, $\beta_1 = 0.85$, $d_{agg} = 20$ mm, $d_s = 10$ mm, $d_b = 20$ mm. Assume two layers of steel.

1- Find, ρ and ρ_{min} : and A_s

Assume steel is yielding, $\phi = 0.9$, $d = h - 90 = 510$ mm.

$$R_n = \frac{M_u}{\phi b d^2} = \frac{300 \times 10^6}{0.9 \times 300 \times 510^2} = 4.272$$

$$\rho = \frac{0.85 f'_c}{f_y} \left(1 - \sqrt{1 - \frac{4 R_n}{1.7 f'_c}} \right) = \frac{0.85 \times 25}{420} \left(1 - \sqrt{1 - \frac{4 \times 4.272}{1.7 \times 25}} \right) = 0.011472$$

$$A_s = \rho b d = 0.011472 \times 300 \times 510 = 1755.15 \text{ mm}^2$$

$$\rho_{min} = \text{Max} \left(\frac{\sqrt{f'_c}}{4 f_y}, \frac{1.4}{f_y} \right) = \text{Max} \left(\frac{\sqrt{25}}{4 \times 420}, \frac{1.4}{420} \right) = \text{Max}(0.00297, 0.00333) = 0.00333$$

$\rho \geq \rho_{min} \rightarrow \checkmark$

2- Find provided area of steel:

$$\# \text{ rebars} = \frac{A_s}{A_b} = \frac{1755.15}{\pi \frac{20^2}{4}} = 5.59 \approx 6 \text{ rebars}$$

$$A_{s \text{ provided}} = \# \text{ rebars} \times A_b = 6 \times \pi \frac{20^2}{4} = 1884.96 \text{ mm}^2$$

3- Check number of layers:

$$S_b = \text{Max} \left(25, d_b, \frac{4}{3} d_{agg} \right) = \text{Max} \left(25, 20, \frac{4}{3} \times 20 \right) = 26.7 \text{ mm}$$

$$n_{max} = \frac{b + S_b - 6 d_s + d_b - 2 \text{cover}}{d_b + S_b} = \frac{300 + 26.7 - 6 \times 10 + 20 - 2 \times 40}{20 + 26.7} = 4.4 \approx 4$$

Six rebars need two layers as assumed. Three rebars in each layer.

4- Find effective depth, d :

$$S_t = \text{Max} \left(25, \frac{4}{3} d_{agg} \right) = \text{Max} \left(25, \frac{4}{3} \times 20 \right) = 26.7 \text{ mm}$$

$$d_t = h - \text{cover} - d_s - \frac{d_b}{2} = 600 - 40 - 10 - 10 = 540 \text{ mm}$$

$$d_{min} = d_t - S - d_b = 540 - 26.7 - 20 = 493.3 \text{ mm}$$

$$d = (d_{min} + d_t) / 2 = 515.65 \text{ mm}$$

5- Check assumption (steel is yielding) at most top steel layer, d_{min}

$$a = \frac{A_s f_y}{0.85 f'_c b} = \frac{6 \times \pi \frac{20^2}{4} \times 420}{0.85 \times 20 \times 250} = 124.185 \text{ mm}$$

$$f'_c = 25 \text{ MPa} < 28 \rightarrow \beta_1 = 0.85$$

$$c = \frac{a}{\beta_1} = \frac{124.185}{0.85} = 146.1 \text{ mm}$$

$$\varepsilon_{min} = \left(\frac{d_{min} - c}{c} \right) \varepsilon_{cu} = \left(\frac{493.3 - 146.1}{146.1} \right) \times 0.003 = 0.0071 \geq \varepsilon_y$$

$0.0071 \geq 0.0021 \rightarrow$ steel is yielding

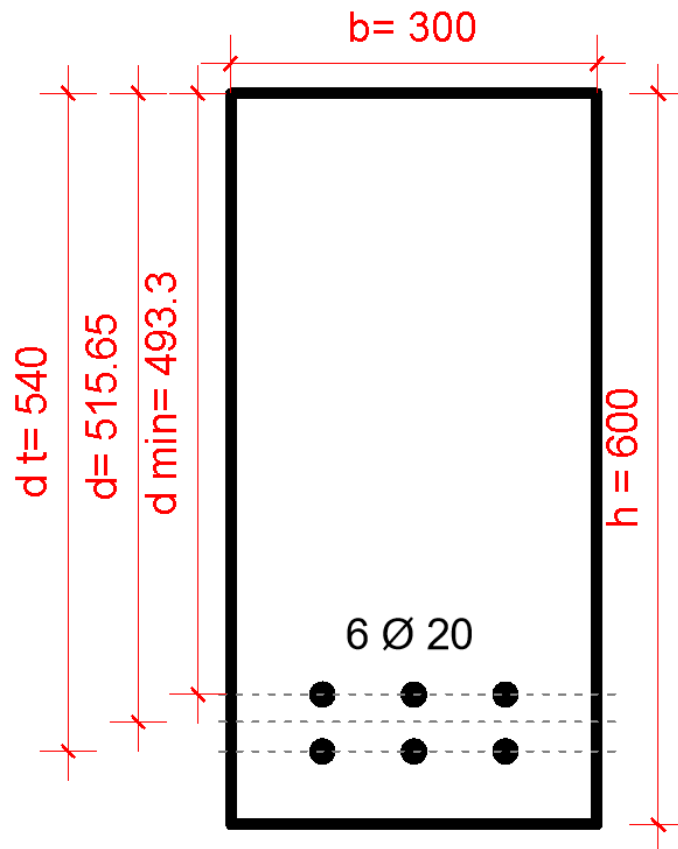
6- Check $\phi M_n \geq M_u$:

Since actual d is greater than assumed $d \rightarrow$ this step is not necessary.

$$\phi M_n = \phi A_s f_y \left(d - \frac{a}{2} \right) = 0.9 \times 6 \times \pi \frac{20^2}{4} \times 420 \left(515.65 - \frac{124.185}{2} \right) = 323.166 \times 10^6 \text{ N.mm}$$

$$\phi M_n = 323.166 \text{ kN.m} \geq M_u = 300 \text{ kN.m}$$

7- Draw beam section:



■■■■

Design of a 375×900 mm rectangular section subjected to an ultimate bending moment $M_u = 1500$ kN.m. Bar / layer spacing is 30 mm. $d_s=10$ mm, $d_b=28$ mm. $f'_c = 35$ MPa, $f_y = 420$ MPa. Assume two layers of steel.

1- Find, ρ and ρ_{min} : and A_s

Assume steel is yielding, $\phi = 0.9$, $d = h - 90 = 810$ mm.

$$R_n = \frac{M_u}{\phi b d^2} = \frac{1500 \times 10^6}{0.9 \times 375 \times 810^2} = 6.774$$

$$\rho = \frac{0.85 f'_c}{f_y} \left(1 - \sqrt{1 - \frac{4R_n}{1.7 f'_c}} \right) = \frac{0.85 \times 35}{420} \left(1 - \sqrt{1 - \frac{4 \times 6.774}{1.7 \times 35}} \right) = 0.01856$$

$$A_s = \rho b d = 0.011472 \times 375 \times 810 = 5637.7 \text{ mm}^2$$

$$\rho_{min} = \text{Max} \left(\frac{\sqrt{f'_c}}{4 f_y}, \frac{1.4}{f_y} \right) = \text{Max} \left(\frac{\sqrt{35}}{4 \times 420}, \frac{1.4}{420} \right) = \text{Max}(0.00352, 0.00333) = 0.00352$$

$\rho \geq \rho_{min} \rightarrow \checkmark$

2- Find provided area of steel:

$$\# \text{ rebars} = \frac{A_s}{A_b} = \frac{5637.7}{\pi \frac{28^2}{4}} = 9.16 \approx 10 \text{ rebars}$$

$$A_{s \text{ provided}} = \# \text{ rebars} \times A_b = 10 \times \pi \frac{28^2}{4} = 6157.5 \text{ mm}^2$$

3- Check number of layers:

$$n_{max} = \frac{b + S_b - 6d_s + d_b - 2cover}{d_b + S_b} = \frac{375 + 30 - 6 \times 10 + 28 - 2 \times 40}{28 + 30} = 5.05 \approx 5$$

ten rebars need two layers as assumed. five rebars in each layer.

4- Find effective depth, d :

$$d_t = h - cover - d_s - \frac{d_b}{2} = 900 - 40 - 10 - 14 = 836 \text{ mm}$$

$$d_{min} = d_t - S - d_b = 836 - 30 - 28 = 778 \text{ mm}$$

$$d = (d_{min} + d_t) / 2 = 807 \text{ mm}$$

5- Check assumption (steel is yielding) at most top steel layer, d_{min}

$$a = \frac{A_s f_y}{0.85 f'_c b} = \frac{6 \times \pi \frac{28^2}{4} \times 420}{0.85 \times 35 \times 375} = 231.813 \text{ mm}$$

$f'_c = 35$ MPa $> 28 \rightarrow \beta_1$ should be calculated

$$\beta_1 = 0.85 - \frac{0.05(f'_c - 28)}{7} = 0.85 - \frac{0.05(35 - 28)}{7} = 0.814$$

$$c = \frac{a}{\beta_1} = \frac{231.813}{0.814} = 284.782 \text{ mm}$$

$$\varepsilon_{min} = \left(\frac{d_{min} - c}{c} \right) \varepsilon_{cu} = \left(\frac{778 - 284.782}{284.782} \right) \times 0.003 = 0.0052 \geq \varepsilon_y$$

$0.0052 \geq 0.0021 \rightarrow$ steel is yielding

6- Check $\phi M_n \geq M_u$:

Since actual d is less than assumed $d \rightarrow$ this step is necessary.

$$\phi M_n = \phi A_s f_y \left(d - \frac{a}{2} \right) = 0.9 \times 10 \times \pi \frac{28^2}{4} \times 420 \left(807 - \frac{231.813}{2} \right) = 1608.55 \times 10^6 \text{ N.mm}$$

$$\phi M_n = 1608.55 \text{ kN.m} \geq M_u = 1500 \text{ kN.m}$$

7- Draw beam section:

