

# Cone Penetration Test (CPT)

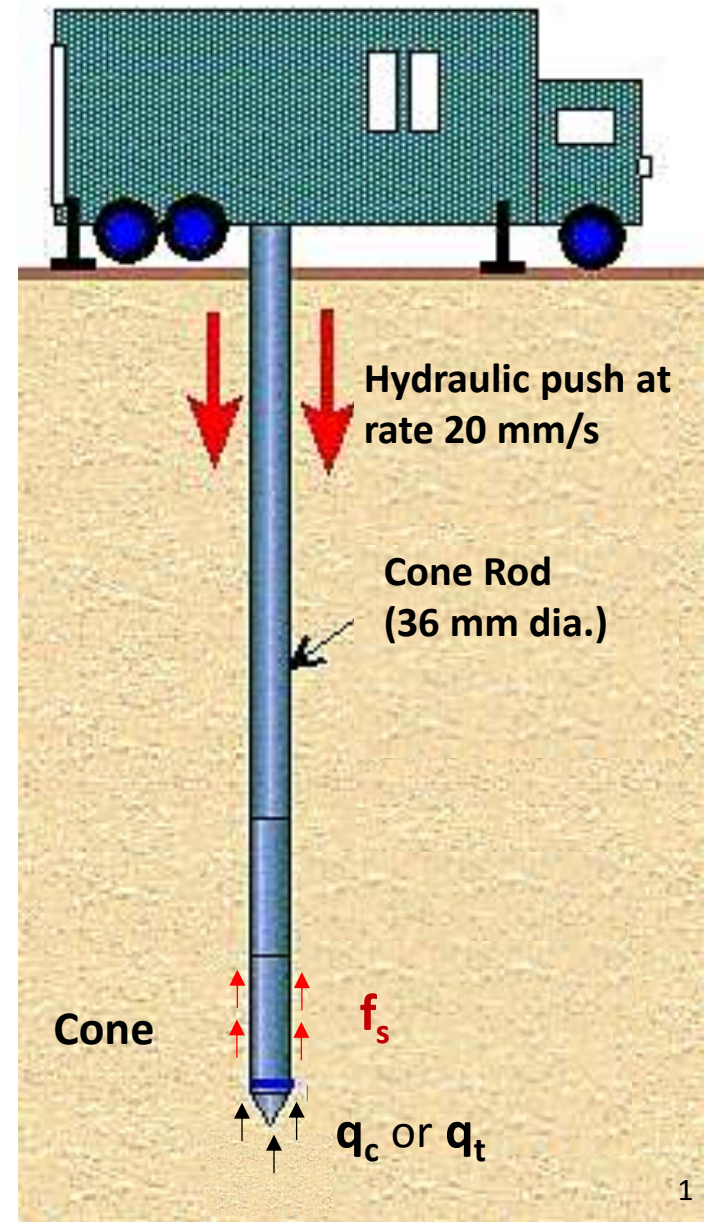
## Definition

- called also "Dutch cone test" or "Static Penetration test".
- The test method consists of pushing an instrumented cone, with the tip facing down, into the ground at a slow controlled rate.
- Cone: 60 degree apex cone, Dia = 36 mm.

## Measures

- Cone or Tip resistance ( $q_c$ ) or ( $q_t$ )
- Sleeve friction ( $f_s$ )
- Water Pore pressure ( $u_b$ )
- Other variables e.g. Shear wave velocity ( $v_s$ )

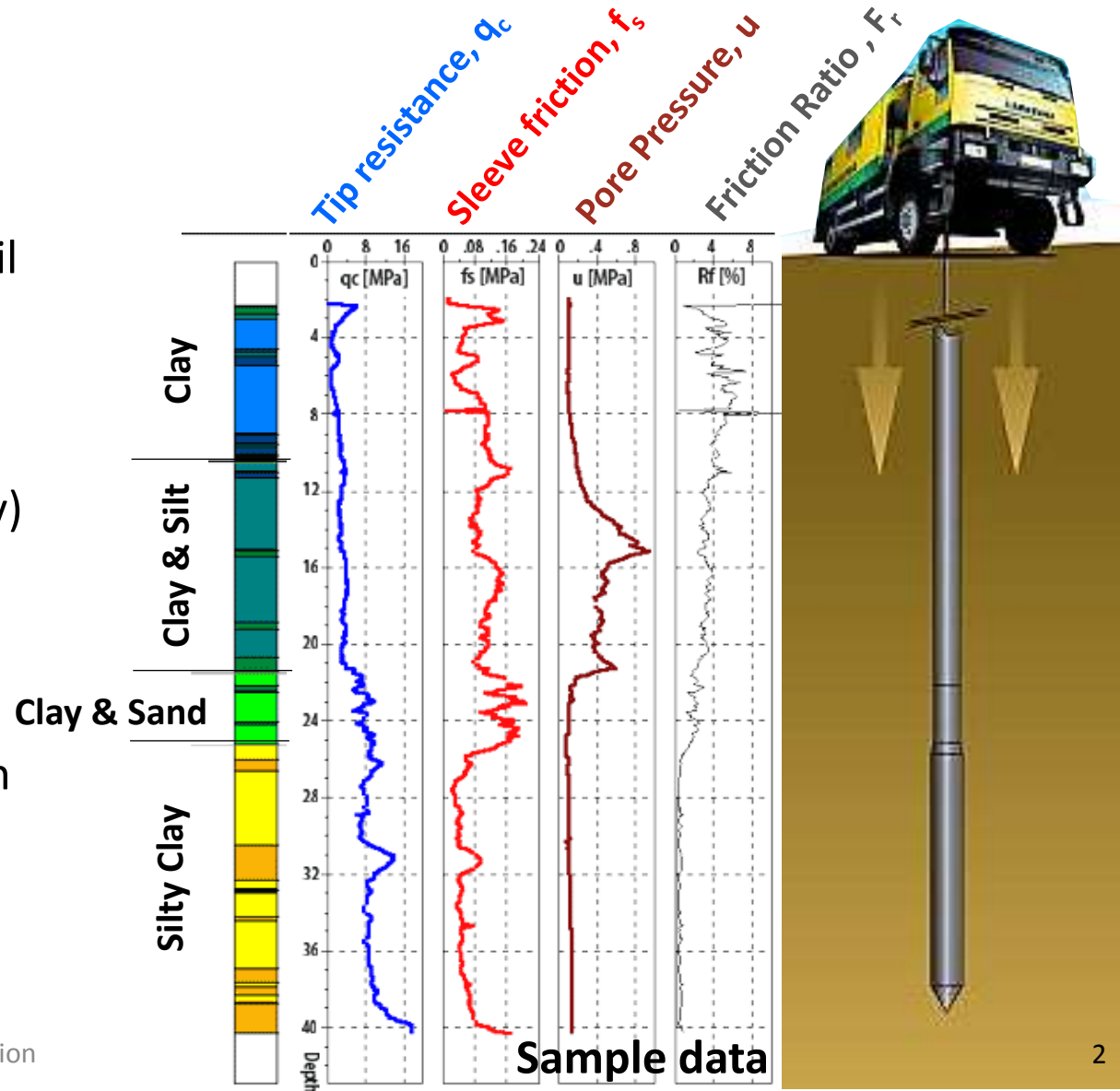
$$\text{Friction Ratio, } F_r = \frac{f_s}{q_c}$$



# Cone Penetration Test (CPT)

## Applications:

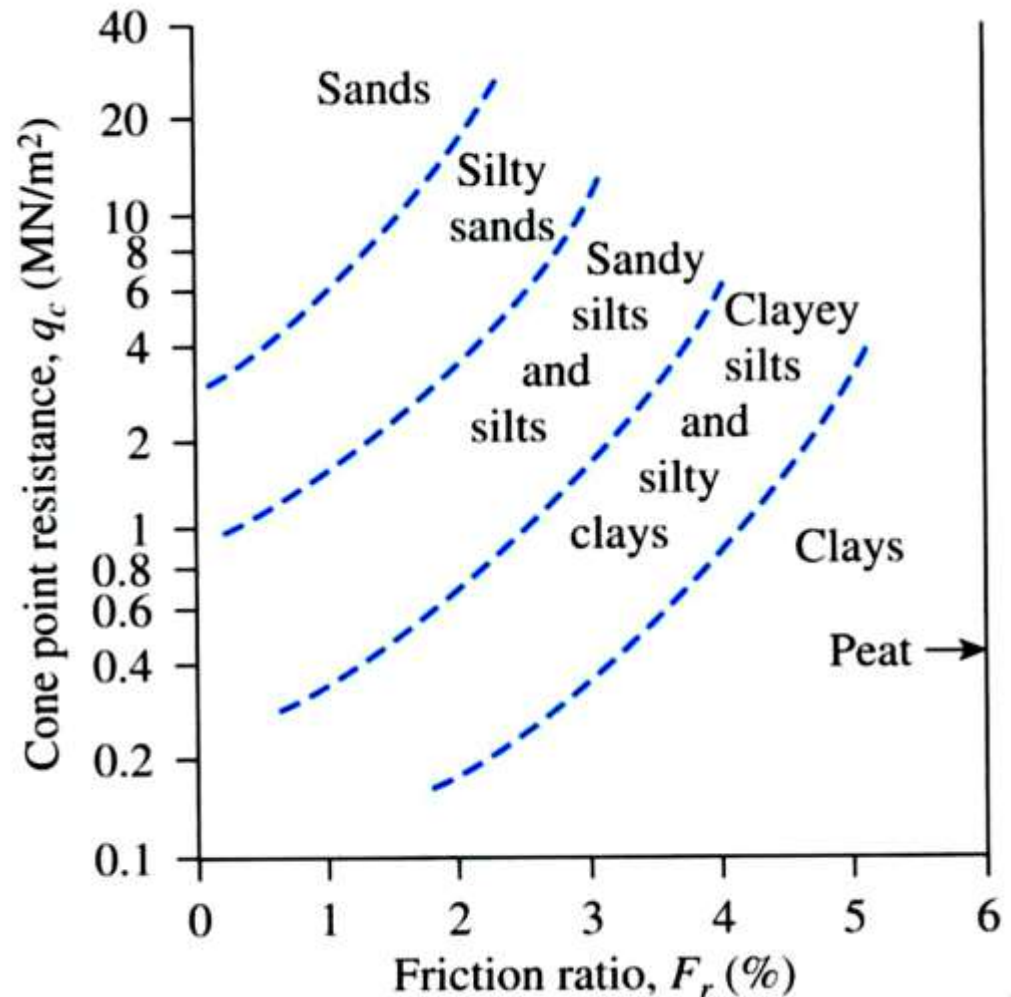
- Soil profile (stratigraphy): soil type identification
- Estimation of geotechnical parameters (strength, compressibility, permeability)
- Evaluation of groundwater conditions (pore pressure)
- Geo-environmental: distribution and composition of contaminants



# Cone Penetration Test (CPT)

## Soil Identification:

- Point resistance  $q_c$ 
  - High in granular soil
  - Low in cohesive soil
- Friction Ratio  $F_r$ 
  - Low in granular soil
  - High in cohesive soil
- However, the cone/tip ( $q_c$ ) and sleeve ( $f_s$ ) resistance increase with increasing overburden stress  $\sigma_0$
- for accurate identification, normalization of  $q_c$  &  $f_s$  by overburden stress is required.



*Classification Chart (Robertson et al., 1983)*

# Cone Penetration Test (CPT)

## Advantages:

- Borehole is not necessary
- Almost continuous data (reading every 10mm)
- Elimination of operator error (automated)
- Reliable, repeatable test results

## Disadvantages:

- Inability to penetrate through gravels and cobbles
- Newer technology = less populated database than SPT
- Lack of sampling



# Cone Penetration Test (CPT)

## Correlation with shear strength

- *In Sand*: the drained friction angle (Ricceri et al's. 2002)

$$\phi' = \tan^{-1} \left[ 0.1 + 0.38 \log \left( \frac{q_c}{\sigma'_o} \right) \right]$$

- *In Clay*: undrained shear strength  $c_u$

$$c_u = \frac{q_c - \sigma_o}{N_K}$$

$$\text{OCR} = 0.37 \left( \frac{q_c - \sigma_o}{\sigma'_o} \right)^{1.01}$$

where:

$q_c$  = the cone (tip) (point) resistance

$\sigma'_o$  &  $\sigma_o$  = effective and total overburden pressure, respectively

$N_K$  = Bearing factor depends on type of cone (varies from 11-20)

OCR = Over Consolidation Ratio



# Cone Penetration Test (CPT)

## Class example: Correlation with shear strength

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In a deposit of normally consolidated dry sand, a cone penetration test was conducted. Following are the results:

| Depth (m) | Point resistance of cone, $q_c$ (MN/m <sup>2</sup> ) |
|-----------|--|
| 1.5       | 2.06   |
| 3.0       | 4.23   |
| 4.5       | 6.01   |
| 6.0       | 8.18   |
| 7.5       | 9.97   |
| 9.0       | 12.42  |

Assuming the dry unit weight of sand to be  $16 \text{ kN/m}^3$ , estimate the average peak friction angle,  $\phi'$ , of the sand. Use equation proposed by Ricceri et al's. 2002.



# Cone Penetration Test (CPT)

Solution:

| Depth, m | $q_c$ (MPa) | $\sigma'_0$ (kPa)    | $q_c/\sigma'_0$    | $\phi'$ (Rad) | $\phi'$ (deg)                      |
|----------|-------------|----------------------|--------------------|---------------|------------------------------------|
| 1.5      | 2.06        | $1.5 \times 16 = 24$ | $2060 / 24 = 85.8$ | 0.69          | $0.69 \times 180 / \pi = 40^\circ$ |
| 3        | 4.23        | 48                   | 88.1               |               |                                    |
| 4.5      | 6.01        |                      |                    |               |                                    |
| 6        | 8.18        |                      |                    |               |                                    |
| 7.5      | 9.97        |                      |                    |               |                                    |
| 9.0      | 12.42       |                      |                    |               |                                    |

$$\phi' = \tan^{-1} \left[ 0.1 + 0.38 \log \left( \frac{q_c}{\sigma'_0} \right) \right]$$

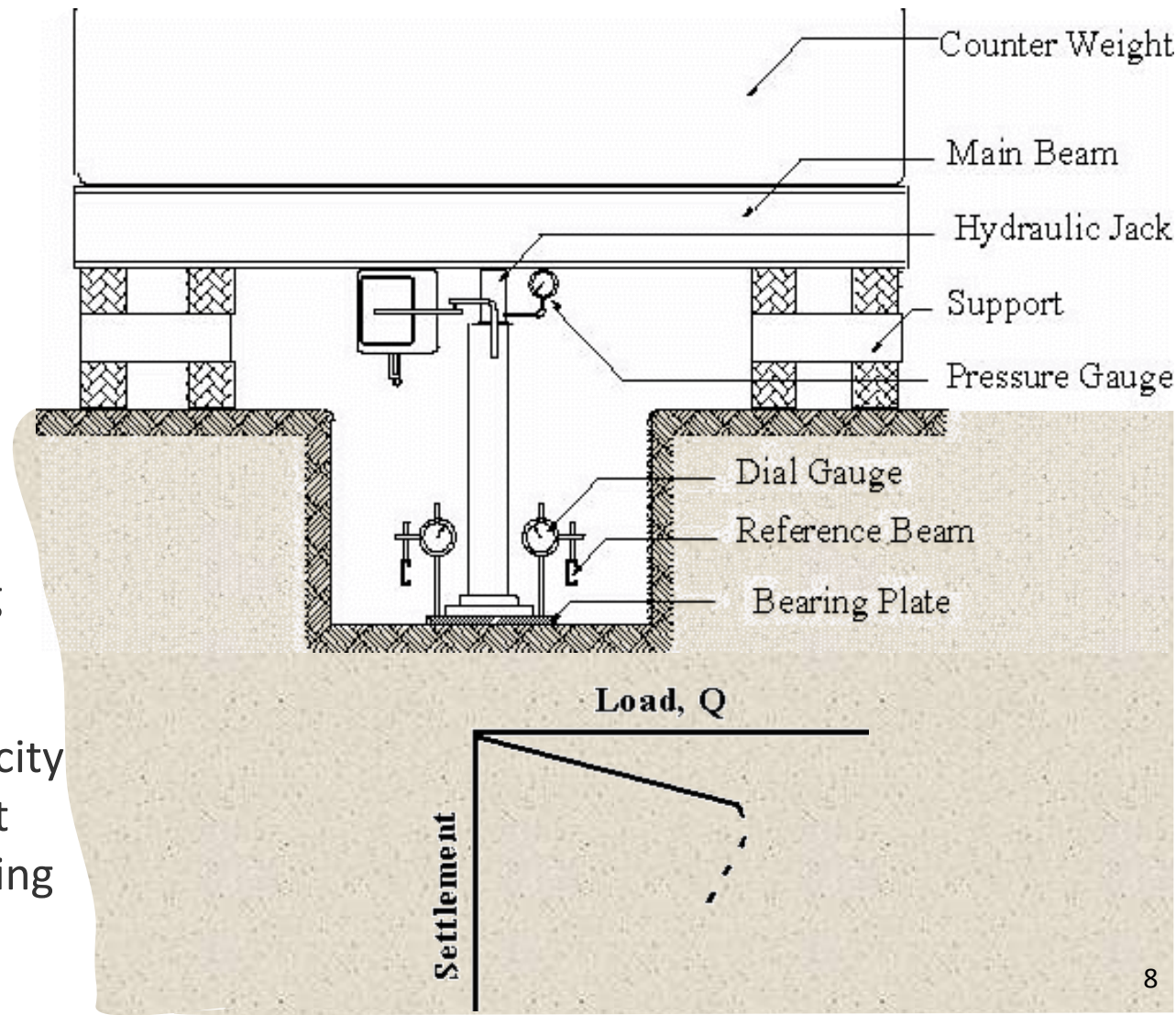
$$\phi'_{av} =$$

$$\phi'_{av} = \Sigma \phi' / 6$$

**Note.**  $\tan^{-1}$  is inverse tangent, the angle returned is in Radian.

# Plate Load Test (PLT)

- Plate load test is a field test to determine the ultimate bearing capacity of soil.
- The test essentially consists in loading a rigid steel plate at the foundation level and determining the settlement corresponding to each load increments.
- The ultimate bearing capacity is then taken as the load at which the plate starts sinking at a rapid rate.





Implementation

▶ Testing

# Laboratory tests





# Laboratory tests

- **Basic physical properties tests (Moisture content, Specific gravity, Soil Indexes, ..)**
- **Particle size test (sieving, Sedimentation)**
- **Direct shear box test**
- **Unconfined compression test**
- **Triaxial test**
- **Consolidation test**
- **Permeability test**
- **Other lab tests: Chemical test (pH, contamination,..)**

# Reporting

- Preparation of Borehole
- Site Investigation Report




# ➤ Preparation of Boring Logs

**Initial information:** Name and address of the drilling company, Driller's name, Job description and reference number, boring information (number, type, and location of, and date of boring).

## Boring Log

Name of the Project Two-story apartment building  
 Location Johnson & Olive St. Date of Boring March 2, 2005  
 Boring No. 3 Type of Boring Hollow-stem auger Ground Elevation 60.8 m

| Soil description   | Depth (m) | Soil sample type and number | $N_{60}$ | $w_n$ (%) | Comments                              |
|--|-----------|-----------------------------|----------|-----------|---------------------------------------|
| Light brown clay (fill)  |           |                             |          |           |                                       |
| Silty sand (SM)  | 1         | SS-1                        | 9        | 8.2       |                                       |
| °G.W.T. <br>3.5 m | 3         | SS-2                        | 12       | 17.6      | LL = 38<br>PI = 11                    |
|  | 4         |                             |          |           |                                       |
| Light gray clayey silt (ML)  | 5         | ST-1                        |          | 20.4      | LL = 36<br>$q_u = 112 \text{ kN/m}^2$ |
|  | 6         | SS-3                        | 11       | 20.6      |                                       |
| Sand with some gravel (SP)   | 7         |                             |          |           |                                       |
| End of boring @ 8 m  | 8         | SS-4                        | 27       | 9         |                                       |

*Example of a typical boring log*

$N_{60}$  = standard penetration number  
 $w_n$  = natural moisture content  
 LL = liquid limit; PI = plasticity index  
 $q_u$  = unconfined compression strength  
 SS = split-spoon sample; ST = Shelby tube sample

Groundwater table observed after one week of drilling

# Preparation of Boring Logs

**Subsurface stratification:** which can be obtained by visual observation of the soil brought out by auger, split-spoon sampler, and thin-walled Shelby tube sampler.

**Groundwater:** Elevation of water table and date observed, use of casing and mud losses, and so on

## Boring Log

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# Preparation of Boring Logs

## Boring Log

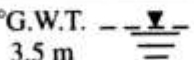
Name of the Project Two-story apartment building

Location Johnson & Olive St. Date of Boring March 2, 2005

Boring No. 3 Type of Hollow-stem auger Ground 60.8 m  
 Boring Elevation

**In-situ tests:** Standard penetration resistance and the depth of SPT

**Samples:** Number, type, and depth of soil sample collected; in case of rock coring, type of core barrel used and, for each run, the actual length of coring, length of core recovery, and RQD.

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## ► Preparation of Boring Logs

### Class example

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The following borehole is part of a site investigation (SI) carried out over a proposed location.

Assess the *subsoil conditions* and *ground-water conditions* based on the borehole data. In particular write about:

- Soil layers: types, description, depth...
- Soil properties: shear strength properties -based on SPT.
- Ground water depth



# ➤ Site Investigation Report

- **When:** After the completion of all of the field and laboratory work, a site investigation report is prepared.
- **Why:** for the use of the design office and for reference during future construction work.
- The report is also called soil exploration report or Geotechnical Factual report.



What should be included in the site investigation report?

# ➤ Site Investigation Report

The report should contain descriptions of the followings:

- **Purpose & Scope** of the investigation
- **Site & Structure:** site location, existing structures, drainage conditions, vegetation,... and information about the structure.
- **Factual Details of field exploration:** boreholes, samples, and testing. For each type, quantities, method, tools should be presented.
- **Geological setting** of the site (variation of depth and thickness of layers as interpreted from the borings)
- **Subsoil and water-table** conditions, (soil parameters as interpreted from the testing results).
- **Design analysis & recommendations:** type of foundation, allowable bearing pressure, settlement estimation, and any special construction procedure; alternatives design solution.
- **Conclusions** and limitations of the investigations

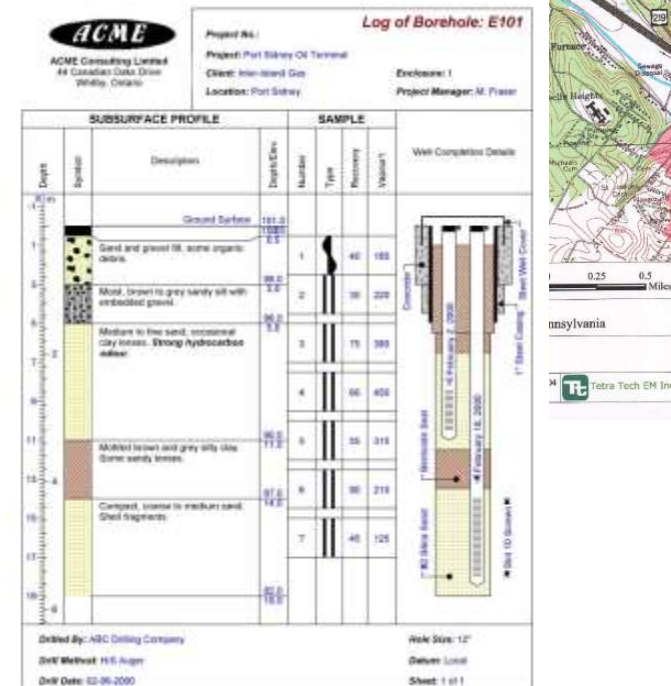
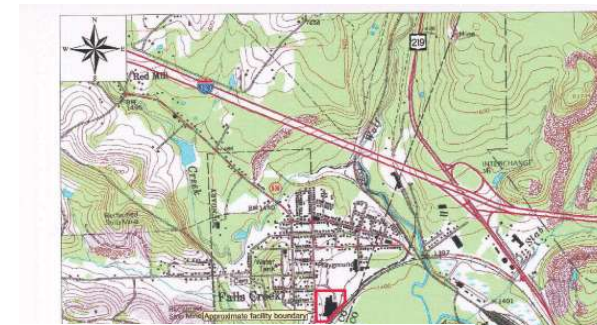
Usually given in another report  
(Geotechnical Design Report)



# Site Investigation Report

The following **graphical presentations** must be attached to the report:

1. General map showing site location
2. A plan view of the location of the borings with respect to the proposed structures and those nearby
3. Boring logs (including in-situ tests results and samples)
4. Laboratory test results
5. Other graphical presentations (geotechnical cross section based on the boring logs, photos of the field work and soil samples,...)



# Site Investigation Report

*Geotechnical cross section based on the boring logs*

